### TECHNICAL MEMORANDUM

### Settler's Hill and Kingsland Drive Feasibility Study

Fabyan Parkway, Settler's Hill Golf Course Entrance to Kingsland Drive

Prepared For:



41W011 Burlington Rd, Campton Hills, IL 60175

Prepared By:



44 S Vail Ave, Suite #201 Arlington Heights, IL 60005

November 2022

### Fabyan Parkway, Settler's Hill Golf Course to Kingsland Drive Technical Memorandum



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#### LIST OF ATTACHMENTS

- 1. Existing Conditions
- 2. Proposed Geometric Concept
- 3. Signal Warrant Analysis
- 4. HCS Reports
- 5. Accident Analysis
- 6. Estimate of Probable Cost



#### Introduction

Based on changes in traffic demand in recent years at the intersections of Settler's Hill and Fabyan Parkway and Kingsland Drive and Fabyan Parkway in Batavia, Illinois, the Kane County Division of Transportation (KDOT) is undertaking a feasibility study to evaluate signal warrants at both intersections, evaluate operational and/or safety deficiencies, and identify proposed alternatives to address these deficiencies. The study will shortlist alternatives to be further vetted in Phase 1. The study will also serve to better define the scope of a subsequent Phase 1 study and facilitate initial coordination with the Forest Preserve District of Kane County, who owns a substantial amount of property on the north side of Fabyan Parkway.

#### **Purpose of Study**

This study will analyze available data to devise a list of potential geometric, signal and access modification options to improve safety, accessibility, and levels of service through the corridor. The study will also generally define the limits of needed improvements. For this analysis, the limits will extend from Raddant Road to the west, Enterprise Avenue to the east and Olympic Drive to the south.

#### **Existing Conditions**

#### Fabyan Parkway and Settler's Hill

Fabyan Parkway is an SRA route. It is typically 4 lanes with a painted center median. The existing right of way is approximately 100 feet. There is curb and gutter on both sides of the road with a mixed-use path along the north side of the road. The entrance to the Settler's Hill golf course is on the north side of the road. This driveway entrance is two lanes with a raised center median. There is an existing temporary traffic signal at this location which is not coordinated with either permanent signal to the east or west.

#### Fabyan Parkway and Kingsland Drive

Fabyan Parkway is an SRA route. There are 4 through lanes, an eastbound right-turn lane, and a painted center median. The right turn lane has an approach taper that is approximately 80 feet long and a turning bay of approximately 75 feet. The existing right of way is approximately 100 feet. There is curb and gutter on both sides of the road with a mixed-use path along the north side of the road. There is a driveway entrance to a landscaping business on the north side of the road.

Kingsland Drive is the south leg of the intersection. It is two lanes with an existing right of way of approximately 50 feet. There is curb and gutter on both sides of the road. Kingsland Drive serves a commercial business district. The radius returns for the southeast and southwest corners of the intersection appear to be undersized. The grass is worn down behind the curb return due to the commercial truck traffic driving over the curb and the grass.

Crash analysis was performed on the area for a 5-year period between 2016 and 2020. Review of the crash information shows that frequent lane shifts to avoid turning vehicles contribute to some types of crashes. See Attachment 5.



#### **Distance Between Intersections**

The distance between the intersection of Fabyan Parkway and Settler's Hill and Fabyan Parkway and Kingsland Drive is approximately 350 feet. The existing conditions are shown in Attachment 1.

#### **Alternatives Analysis**

#### Fabyan Parkway and Settler's Hill

Three different alternatives were considered:

#### No-Build Alternative

The No-Build alternative consists of not changing anything at this location. The existing temporary signal is not warranted under current conditions. The signal is not part of the interconnected system within the corridor and impedes its operations.

#### **Build Alternative A**

Build Alternative "A" involves removing the temporary signal at Settler's Hill. The two intersections would be maintained as separate intersections with no geometric modifications at either intersection. This option would not address operational and safety deficiencies at the intersections.

#### **Build Alternative B**

Build Alternative "B" would remove the temporary signal at Settler's Hill and the two intersections would be maintained as separate. The two intersections would have stop control for Setter's Hill and Kingsland Drive with turn lane channelization improvements.

#### Fabyan Parkway and Kingsland Drive

Build Alternative "B" would remove the temporary signal at Settler's Hill and the two intersections would be maintained as separate. The two intersections would have stop control for Setter's Hill and Kingsland Drive with turn lane channelization improvements. The turning radii for trucks turning to and from Kingsland Drive would be improved.

#### Combining the Two Intersections

Two different alternatives were considered:

#### **Build Alternative C**

Build Alternative "C" consists of removing the temporary signal and shifting the golf course driveway east to align with Kingsland Drive and providing turn movement channelization. The single intersection would have stop control for Settler's Hill and Kingsland Drive. The turning radii for trucks turning to and from Kingsland Drive would be improved. This would have environmental and property impacts on the golf course and the landscape company. These impacts would outweigh the resulting benefits of aligning the two accesses.

#### **Build Alternative D**

Build Alternative "D" consists of adding a new signal to Alternative C. Based on redistributed existing volumes, the signal would not be warranted.



#### Recommendations

The signal warrant analysis concluded that Signals are not warranted at either intersection under existing or proposed conditions for any alternative. See Attachment 3. All intersections perform at acceptable Level of Service under existing and projected conditions. See Attachment 4. There are high delays on east and west approaches that can be alleviated with the addition of turning lanes. Crash analysis was performed on the area for a 5-year period between 2016 and 2020. There does not appear to be a correlation between pavement condition and roadway lighting condition with any one type of crash. See Attachment 5.

P-C analyzed four alternative geometries and recommends Alternative B. A concept of the alternative was presented to the County along with a discussion of pros and cons. The alternative features the removal of the traffic signal and provides turning movement channelization to improve safety and operations. The concept of the alternative is attached. See Attachment 2. The widening accommodating added channelization was asymmetrical to the north to avoid potential wetlands along the south parkway. The results of a wetland identification and delineation study may require adjustments to the conceptual geometry.

An engineer's opinion of probable cost of construction was developed for Alternative B. This cost does not include right of way acquisition, permit fees, storm water management modifications, nor engineering costs. The cost is \$762,015.50. See Attachment 6.

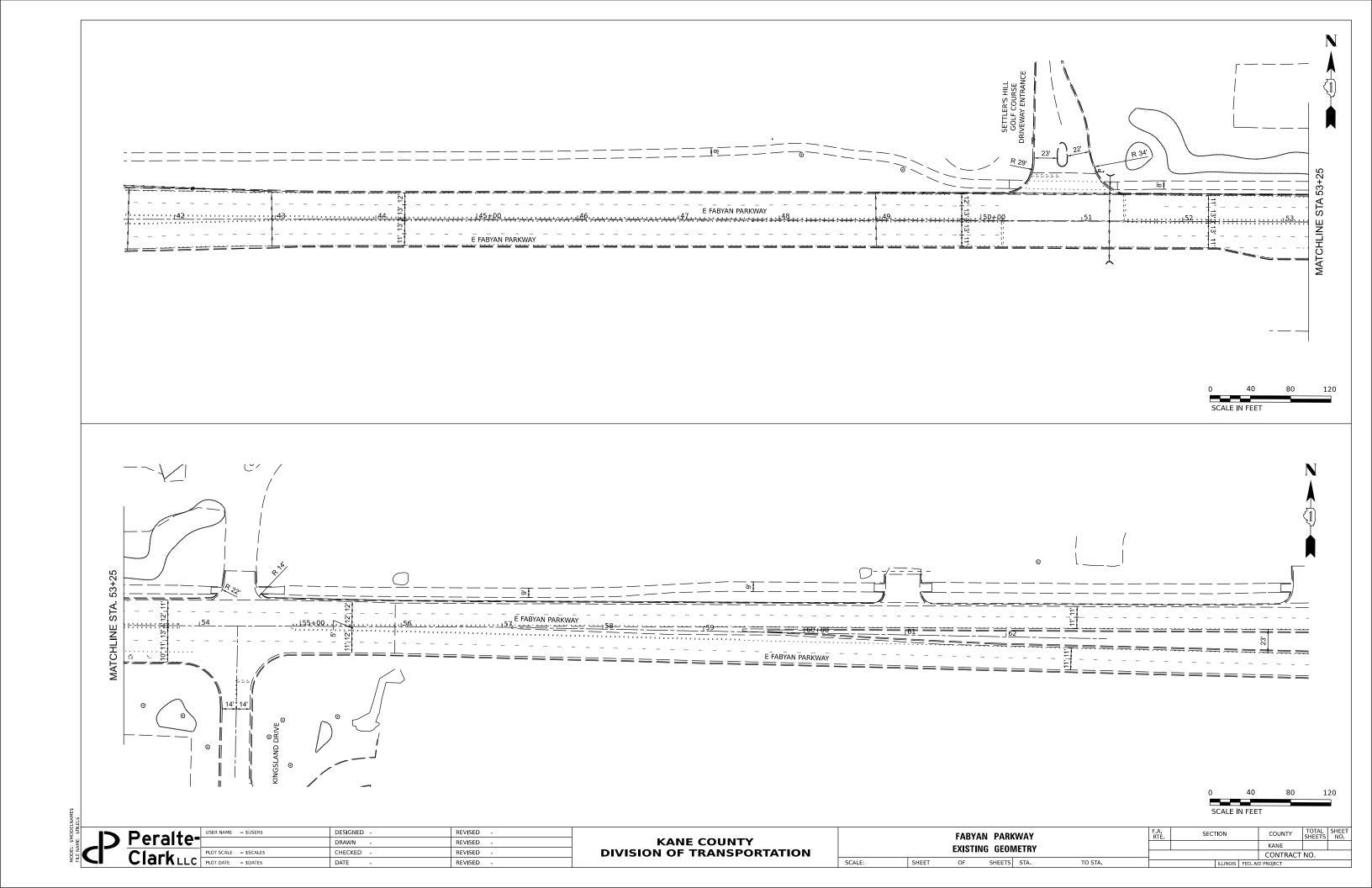
#### **Next Steps**

Since a feasible option is selected as part of this study process, potential environmental impacts must be evaluated in the following steps. Key issues would include the following:

- Wetland impacts
- Right of way acquisition investigation
- Traffic signal removal
- Storm water management modifications
- Section 6f processing
- Adherence to clear zone guidelines
- ADA accommodations for the relocated multi-use path

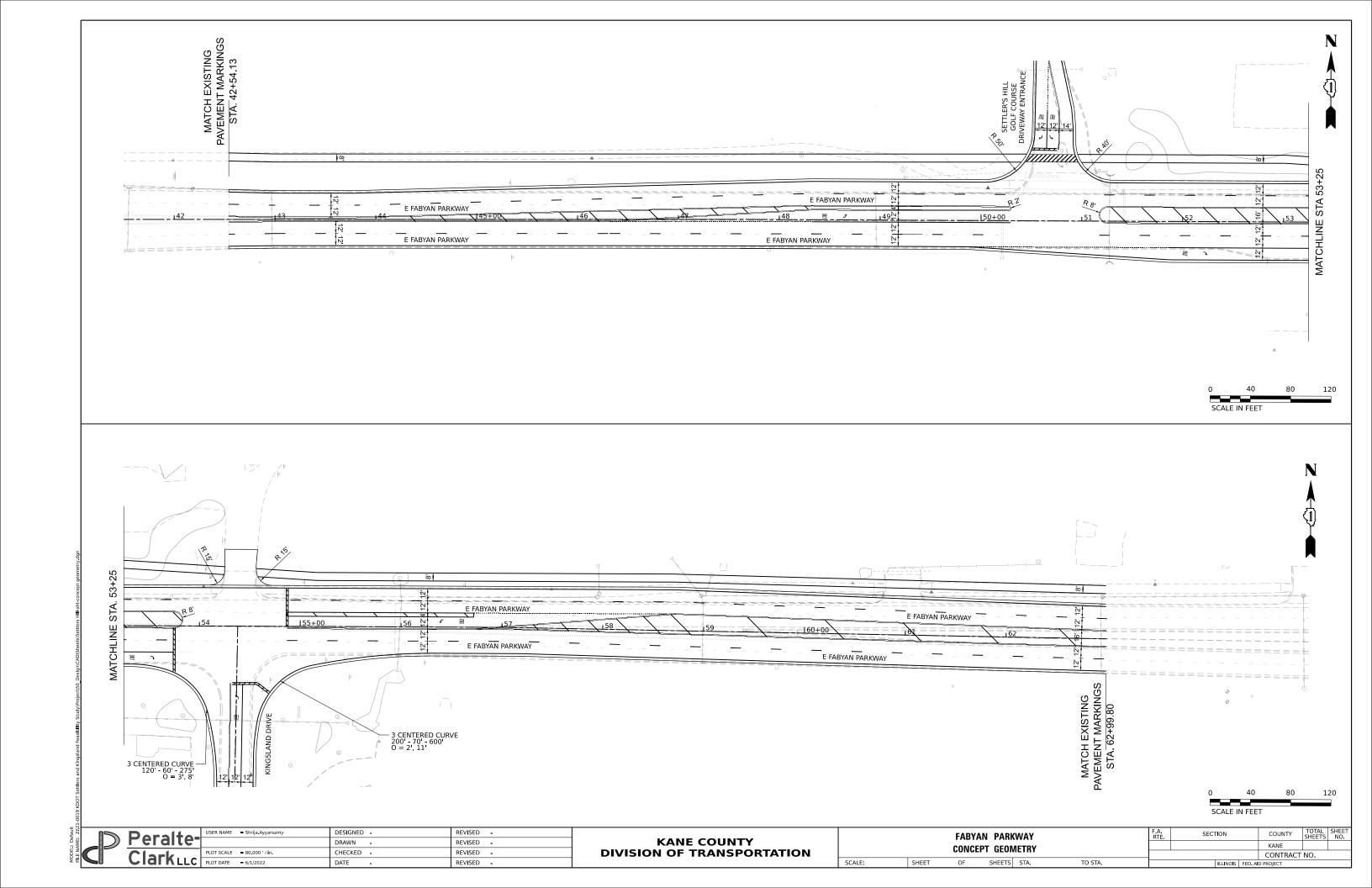


**ATTACHMENT 1: Existing Conditions** 





#### ATTACHMENT 2: Proposed Geometric Concept





#### **ATTACHMENT 3: Signal Warrants Analysis**



#### Signal Warrants Analysis

#### • <u>Settler's Hill</u>

- Not warranted
- o Minor street volumes very low during peak hours (less than 20)
- o 5 accidents in 5 years through 2020 (Most severe: Minor injury (B))

#### • Kingsland Drive

- Not warranted
- Warrant 1 met for 2 hours
- o 10 accidents in 5 years through 2020 (Most severe: Minor injury (B))

#### • <u>Proposed Combined Intersection</u>

- Not Warranted
- o Minor street adjusted volume less than 80
- o 15 accidents in 5 years through 2020

## STATE OF ILLINOIS KANE COUNTY DISTRICT #1, BUREAU OF TRAFFIC

#### **SUMMARY OF TRAFFIC SURVEY**

INTERSECTION: Settler's Hill Golf Course Driveway and E Fabyan Parkway

MUNICIPALITY: Batavia, IL COUNTY: Kane

	TF	RAFFIC FRO	OM NOR	TH	TR	AFFIC FR	OM SOL	JTH		T	RAFFIC F	ROM EAS	ST	TR	RAFFIC F	ROM WE	ST		
ROUTE:		Hill Golf C		SRA	N.A.			SRA			n Parkwa			E Fabyar			✓ SRA		_
	N. OF:	E Fabyan	Parkway		S. OF:	N.A.				E. OF:	Settler's	Hill Golf	Course	W. OF:		Hill Golf	Course		
		GOING				GOING			TOTAL		GOING				GOING			TOTAL	
	EAST	SOUTH	WEST		WEST	NORTH	EAST		NORTH	SOUTH	WEST	NORTH		NORTH	EAST	SOUTH		EAST	
START					←	<b>↑</b>	_		AND		←	<b>^</b>		<b>↑</b>	<b>→</b>	-1		AND	GRAND
HOUR	<b></b>	•	<b>←</b>	TOTAL		ı	ı	TOTAL	SOUTH	<b>V</b>			TOTAL			<b>V</b>	TOTAL	WEST	TOTAL
6:00	5	0	12	17	0	0	0	0	17	0	343	5	348	14	817	0	831	1179	1196
7:00	3	0	5	8	0	0	0	0	8	0	561	9	570	17	986	0	1003	1573	1581
8:00	8	0	10	17	0	0	0	0	17	0	549	7	556	10	702	0	712	1268	1285
9:00	8	0	11	19	0	0	0	0	19	0	491	9	500	8	534	0	542	1042	1061
10:00	9	0	11	19	0	0	0	0	19	0	465	8	473	10	458	0	468	941	960
11:00	10	0	9	19	0	0	0	0	19	0	545	10	555	10	495	0	505	1060	1079
12:00	9	0	12	21	0	0	0	0	21	0	586	11	597	8	501	0	509	1106	1127
13:00	9	0	10	18	0	0	0	0	18	0	590	12	602	11	580	0	591	1193	1211
14:00	10	0	11	21	0	0	0	0	21	0	739	11	750	7	645	0	652	1403	1424
15:00	11	0	8	19	0	0	0	0	19	0	1071	10	1081	6	626	0	632	1713	1732
16:00	11	0	15	26	0	0	0	0	26	0	1103	10	1113	19	595	0	614	1727	1753
17:00	12	0	7	18	0	0	0	0	18	0	972	10	982	7	599	0	606	1588	1606
18:00	10	0	8	18	0	0	0	0	18	0	594	10	604	6	409	0	415	1020	1038
19:00	10	0	7	17	0	0	0	0	17	0	396	9	405	7	296	0	303	708	725
20:00	10	0	7	17	0	0	0	0	17	0	284	9	293	6	193	0	199	493	510
21:00	10	0	7	17	0	0	0	0	17	0	228	9	237	7	174	0	181	418	435

Trip Hourly distribution is even except for Peak hours whose Trip gen nos are different.

#### **REVIEW INFORMATION**

COUNTS USED: QUALITY COUNTS

COUNT DATE(S): 03.29.202 - AM 03.29.202 - PM

DATE REVIEWED: 04.29.2022 REVIEWED BY: Shrija Ayyarsamy

### STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF TRAFFIC

DISTRICT #1

#### **RIGHT TURN FACTORIZATION SHEET**

INTERSECTION: Settler's Hill Golf Course Driveway @ E Fabyan Parkway

MUNICIPALITY: Batavia, IL COUNTY: Kane

		(	MINOR S STREET N CONFIG. #: VOLU	Settler's	Hill Golf	MAINLINE APPROACH	BASE RIGHT TURN	MAINLINE CONGESTION	ADJUSTED RIGHT TURN	ADJUSTED RIGHT	ADJUSTED MINOR
DIR	HOUR BEGIN	L	T THROUGH	R RIGHT	A TOTAL	VOLUME PER LANE	REDUCTION %	FACTOR %	REDUCTION %	TURNS	STREET VOLUMES
DIK	DEGIN	LEFI	THROUGH	RIGHT	TOTAL		70	70	70		
	6:00	5	0	12	17	172	75%	0%	75%	3	8
	7:00	3	0	5	8	281	75%	0%	75%	1	4
	8:00	8	0	10	17	275	75%	0%	75%	3	10
	9:00	8	0	11	19	246	75%	0%	75%	3	11
	10:00	9	0	11	19	233	75%	0%	75%	3	11
	11:00	10	0	9	19	273	75%	0%	75%	2	12
	12:00	9	0	12	21	293	75%	0%	75%	3	12
	13:00	9	0	10	18	295	75%	0%	75%	3	11
	14:00	10	0	11	21	370	75%	0%	75%	3	13
	15:00	11	0	8	19	536	75%	10%	65%	3	14
	16:00	11	0	15	26	552	75%	10%	65%	5	16
	17:00	12	0	7	18	486	75%	5%	70%	2	13
	18:00	10	0	8	18	297	75%	0%	75%	2	12
	19:00	10	0	7	17	198	75%	0%	75%	1	11
	20:00	10	0	7	17	142	75%	0%	75%	2	12
	21:00	10	0	7	17	114	75%	0%	75%	1	19

#### **Lane Configurations**



														BASE
LEFT	THROUGH	RIGHT	TOTAL (A)	.7A	.35A	3T	T/3	(T+L)	(T+R)	3R	3L	T/2	T/4	REDUCTION
5.111	0	12	17	12	6	0	0	5	12	37	15	0	0	75%
2.538	0	5	8	6	3	0	0	3	5	16	8	0	0	75%
7.586	0	10	17	12	6	0	0	8	10	29	23	0	0	75%
8.28	0	11	19	14	7	0	0	8	11	33	25	0	0	75%
8.708	0	11	19	14	7	0	0	9	11	32	26	0	0	75%
10.06	0	9	19	14	7	0	0	10	9	28	30	0	0	75%
9.319	0	12	21	15	7	0	0	9	12	36	28	0	0	75%
8.717	0	10	18	13	6	0	0	9	10	29	26	0	0	75%
10.23	0	11	21	15	7	0	0	10	11	33	31	0	0	75%
10.91	0	8	19	14	7	0	0	11	8	25	33	0	0	75%
10.7	0	15	26	18	9	0	0	11	15	46	32	0	0	75%
11.69	0	7	18	13	6	0	0	12	7	20	35	0	0	75%
10.24	0	8	18	13	6	0	0	10	8	24	31	0	0	75%
9.892	0	7	17	12	6	0	0	10	7	22	30	0	0	75%
10.29	0	7	17	12	6	0	0	10	7	21	31	0	0	75%
9.804	0	7	17	12	6	0	0	10	7	22	29	0	0	75%

#### REVIEW INFORMATION

MAINLINE CONGE	STION FACTORS
VOLUMES	FACTOR (%)
0-399	0
400-499	5
500-599	10
600-699	15
700-799	20
800-899	25
900-999	30
1000-1099	35
1100-1199	40
1200-1299	45
1300-1399	50
1400-1499	55

COUNTS USED: QUALITY COUNTS

COUNT DATE(S): 03.29.2022

DATE REVIEWED: 04.29.2022

REVIEWED BY: Shrija Ayyarsamy

### **SIGNAL WARRANT REVIEW SHEET**

			IL	LINOIS DE	PARTMENT	OF TRAN	SPORTATIO	ON		SRA :	Faby	/an Pa	arkw	ay
												Yes	)	No
		Settler's Hill					_	County:	Kane			_		
	Municipality:	Batavia, Illin	nois		_									
	-	nit of Major Route	45 mph		Isola		ty with Populati		N	-				
Nu	mber of Lanes of	n Major approach	2			Number of	f Lanes on Min	or approach	2	-				
									WARR	ANT 1	1	Vaa		No
	Major Street	Adj. Minor		RANT 1	HICH MEET THE F	7: 8 hrs of one					£ 41 £-11	Yes		No
	Volume (both	Street Volume (higher volume			WARRAN	7. 8 hrs of one	of the Following:		Warrant 1 is i Condit		of the follow	ving Condi	Yes	met: No
HOUR BEGIN	approaches)	approach)	A 100%	B 100%	80% of A	80% of B	80% of Warr #4			HICULAR VO	LUME		163	140
6:00	1179	8	10070	100%	0070 0171	007/012	00% 01 11411 # 1		<ul><li>Condit</li></ul>	ion B			Yes	No
7:00	1573	4								ON OF CONT	INUOUS TRA	AFFIC		
8:00	1268	10						€	•			0 hours	Yes	No
9:00	1042	11						(SRA)						
10:00	941	11						3LE			]	Yes		No
11:00	1060	12						CAE			_			
12:00	1106	12						PLI						
13:00	1193	11						NOT APPLICABLE				Yes		No
14:00	1403	13						5	PEAK-HOUR VO	LUME	_			
15:00	1713	14												
16:00	1727	16							WARR	ANT 4		Yes		No
17:00	1588	13						F	PEDESTRIAN VO	DLUME	•			
18:00	1020	12												
19:00	708	11						Ī	WARR	ANT 5		Yes		No
20:00	493	12						\$	SCHOOL CROS	SING				
21:00	418	19						_			_			
			•	·			·		WARR			Yes		No
Hours M	et:		0 Hours	0 Hours	0 Hours	0 Hours			COORDINATED	SIGNAL SYS	ΓEM			
Volume I	Requirements	: MAJOR:	600	900	480	720		. <u>-</u>						
		MINOR:	200	150	160	120	_		WARR			Yes		No
				Increased Co	ondition B Mind	oi Increased C	ondition B Mine	or Street Vol	ACCIDENT EXP	ERIENCE	1	1	ı	1
Reviev	v Informatio	n				_			YEAR:	2016	2017	2018	2019	2020
		QUALITY CO	UNTS					TOTAL NUMBER OF	F ACCIDENTS:	2	3			-
	Count Date(s):							BER CORRECTABLE		0	0			
D		: 04.29.2022 : Shrija Ayyars	nomy.					D LESS RESTRICTIV				No		
	Reviewed by	. Ollija Ayyais	Salliy				ARE	E VOLUME REQUIRE	MENTS MET?	-		No		
								I	WARR	ANT 8		Yes		No
								F	ROADWAY NET	WORK				
Comm	ents													
						-		Ī	WARR	ANT 9		Yes		No
								ī	ntersection Near	a Grade Cros	sing			
									P OR YIELD ( WITH GRAD				NORTH	1
										storage d				
												#	%	Adj. Factor
									RAIL	TRAFFIC	PER DAY =			i acioi
								HIGH O	CCUPANCY I	BUSSES PE				

OVERALL ADJUSTMENT FACTOR =

## STATE OF ILLINOIS KANE COUNTY DISTRICT #1, BUREAU OF TRAFFIC

#### **SUMMARY OF TRAFFIC SURVEY**

INTERSECTION: Kingsland Dr @ E Fabyan Parkway

MUNICIPALITY: Batavia, IL COUNTY: Kane

	TF	RAFFIC FRO					OM SOL	JTH		TI	RAFFIC F	ROM EAS	ST	TR	AFFIC F	ROM WE	ST		
ROUTE:	Kingslar	nd Dr		SRA	Kingslar	nd Drive		SRA		E Fabya	n Parkwa	ıy	✓ SRA	E Fabyar			✓ SRA		
	N. OF:	E Fabyan	Parkway		S. OF:	E Fabyar	n Parkwa	ay		E. OF:	Kingslar	nd Drive		W. OF:	Kingslaı	nd Drive			
		GOING				GOING			TOTAL		GOING				GOING			TOTAL	
	EAST	SOUTH	WEST		WEST	NORTH	EAST		NORTH	SOUTH	WEST	NORTH		NORTH	EAST	SOUTH		EAST	
START					←	<b>↑</b>	_		AND		←	<b>†</b>			<b>→</b>			AND	GRAND
HOUR	<b></b>	•	<b>←</b>	TOTAL	l	'		TOTAL	SOUTH	<b>V</b>			TOTAL		·	<b>V</b>	TOTAL	WEST	TOTAL
6:00	4	2	1	7	7	1	18	26	33	45	334	20	399	3	763	53	819	1218	1251
7:00	3	0	0	3	13	0	18	31	34	43	550	0	593	1	927	60	988	1581	1615
8:00	1	0	1	2	13	0	19	32	34	28	540	2	570	0	675	29	704	1274	1308
9:00	5	0	1	6	13	0	28	41	47	21	479	1	501	2	507	25	534	1035	1082
10:00	3	0	0	3	13	0	17	30	33	11	459	2	472	1	450	8	459	931	964
11:00	2	0	0	2	35	0	31	66	68	23	510	3	536	3	481	24	508	1044	1112
12:00	0	0	2	2	36	0	68	104	106	31	551	2	584	0	497	41	538	1122	1228
13:00	3	0	0	3	22	0	34	56	59	25	568	5	598	2	549	31	582	1180	1239
14:00	0	0	1	1	38	0	103	141	142	36	699	0	735	1	619	29	649	1384	1526
15:00	3	0	0	3	42	0	96	138	141	17	1031	8	1056	1	604	18	623	1679	1820
16:00	10	0	5	15	43	0	81	124	139	9	1063	1	1073	0	595	6	601	1674	1813
17:00	2	1	0	3	16	0	56	72	75	15	958	0	973	0	587	11	598	1571	1646
18:00	0	0	0	0	7	0	21	28	28	6	586	0	592	0	405	5	410	1002	1030
19:00	0	0	0	0	10	0	9	19	19	4	386	0	390	0	287	6	293	683	702
20:00	0	0	0	0	7	0	6	13	13	1	276	0	277	0	188	3	191	468	481
21:00	0	0	0	0	4	0	3	7	7	1	224	0	225	0	171	1	172	397	404

Note 1

#### REVIEW INFORMATION

Note 1 Minor Leg - High Volume Approach

COUNTS USED: QUALITY COUNTS

COUNT DATE(S): 03.29.202 - AM 03.29.202 - PM

DATE REVIEWED: 04.29.2022 REVIEWED BY: Shrija Ayyarsamy

### STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF TRAFFIC

DISTRICT #1

#### **RIGHT TURN FACTORIZATION SHEET**

#### INTERSECTION: Kingsland Drive @ E Fabyan Parkway

MUNICIPALITY: Batavia, IL COUNTY: Kane County

		CONFIG. #: 1 VOLUMES									
		(	STREET N Kingsland Drive   CONFIG. #:1   VOLUMES   L		d Drive	CRITICAL MAINLINE APPROACH	TORN	CONGESTION	ADJUSTED RIGHT TURN	ADJUSTED RIGHT	ADJUSTED MINOR
DIR	HOUR BEGIN	L	· · · · ·	i	A TOTAL	VOLUME PER LANE	REDUCTION %	FACTOR %	REDUCTION %	TURNS	STREET VOLUMES
	6:00	7	1	18	26	382	40%	0%	40%	11	19
	7:00	13	0	18	31	464	40%	0%	40%	11	24
	8:00	13	0	19	32	338	40%	0%	40%	11	24
	9:00	13	0	28	41	254	40%	0%	40%	17	30
	10:00	13	0	17	30	225	40%	0%	40%	10	23
	11:00	35	0	31	66	241	40%	0%	40%	19	54
	12:00	36	0	68	104	249	40%	0%	40%	41	77
	13:00	22	0	34	56	275	40%	0%	40%	20	42
	14:00	38	0	103	141	310	60%	0%	60%	41	79
	15:00	42	0	96	138	302	40%	10%	30%	67	109
	16:00	43	0	81	124	298	40%	10%	30%	57	100
	17:00	16	0	56	72	294	60%	5%	55%	25	41
	18:00	7	0	21	28	203	60%	0%	60%	8	15
	19:00	10	0	9	19	144	40%	0%	40%	5	15
	20:00	7	0	6	13	94	40%	0%	40%	4	11
	21:00	4	0	3	7	86	40%	0%	40%	2	6

Note 2 Note 1

West Leg i Base reduction factor based On Lane configuration 1

#### REVIEW INFORMATION

MAINLINE CONGES	STION FACTORS
VOLUMES	FACTOR (%)
0-399	0
400-499	5
500-599	10
600-699	15
700-799	20
800-899	25
900-999	30
1000-1099	35
1100-1199	40
1200-1299	45
1300-1399	50
1400-1499	55

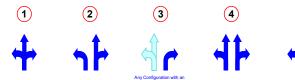
COUNTS USED: QUALITY COUNTS

COUNT DATE(S): 03.29.2022

DATE REVIEWED: 04.29.2022

REVIEWED BY: Shrija Ayyarsamy

#### **Lane Configurations**



BASE														
REDUCTION	T/4	T/2	3L	3R	(T+R)	(T+L)	T/3	3T	.35A	.7A	TOTAL (A)	RIGHT	THROUGH	LEFT
40%	0	1	21	54	19	8	0	3	9	18	26	18	1	7
40%	0	0	39	54	18	13	0	0	11	22	31	18	0	13
40%	0	0	39	57	19	13	0	0	11	22	32	19	0	13
40%	0	0	39	84	28	13	0	0	14	29	41	28	0	13
40%	0	0	39	51	17	13	0	0	11	21	30	17	0	13
40%	0	0	105	93	31	35	0	0	23	46	66	31	0	35
40%	0	0	108	204	68	36	0	0	36	73	104	68	0	36
40%	0	0	66	102	34	22	0	0	20	39	56	34	0	22
60%	0	0	114	309	103	38	0	0	49	99	141	103	0	38
40%	0	0	126	288	96	42	0	0	48	97	138	96	0	42
40%	0	0	129	243	81	43	0	0	43	87	124	81	0	43
60%	0	0	48	168	56	16	0	0	25	50	72	56	0	16
60%	0	0	21	63	21	7	0	0	10	20	28	21	0	7
40%	0	0	30	27	9	10	0	0	7	13	19	9	0	10
40%	0	0	21	18	6	7	0	0	5	9	13	6	0	7
40%	0	0	12	9	3	4	0	0	2	5	7	3	0	4

#### Note 3

able for Main	line cong	1		
R	Α			
RIGHT	TOTAL	R/A		Base Right Turn reductio
18	26		0.69	0.4
18	31		0.58	0.4
19	32		0.59	0.4
28	41		0.68	0.4
17	30		0.57	0.4
31	66		0.47	0.4
68	104		0.65	0.4
34	56		0.61	0.4
103	141		0.73	0.6
96	138		0.70	0.4
81	124		0.65	0.4
56	72		0.78	0.6
21	28	]	0.75	0.6
9	19		0.47	0.4
6	13		0.46	0.4
3	7	]	0.43	0.4

#### NOT On

the minor street, the higher volume shall not be required to be on the same approach during each of these 8 hours.

### **SIGNAL WARRANT REVIEW SHEET**

ILLINOIS DEPARTMENT OF TRANSPORTATION

SRA : E Fabyan Pakrway
Yes No

	Intersection: Municipality:	Kingsland D Batavia, IL	rive @ E	Fabyan P	arkway		_	County	/: Kane	Coun	ty			
	Speed Lir Number of Lanes o	-	45 mph 2		Isolat	ted Community Number of	y with Populati Lanes on Min			- -				
			CHEC	CK ANY HOURS W	HICH MEET THE F	FOLLOWING WAR	RANTS		WARR	ANT 1	1	Yes		No
	Major Street Volume (both	Adj. Minor Street Volume	WARF	RANT 1	WARRANT	7: 8 hrs of one of	of the Following:		Warrant 1 is	met if any o	of the follow	ing Condi	tions are	met:
HOUR BEGIN	approaches)	(higher volume approach)	A 100%	B 100%	80% of A	80% of B	80% of Warr #4		Condit     MINIMUM VE	ion A EHICULAR VO	DLUME		Yes	No
6:00	1218	19							<ul> <li>Condit</li> </ul>				Yes	No
7:00	1581	24							INTERRUPT	ION OF CON	TINUOUS TRA	FFIC		
8:00	1274	24						NOT APPLICABLE (SRA)	•			0 hours	Yes	No
9:00	1035	30						В (S			,			
10:00	931	23						E E				Yes		No
11:00	1044	54	l ,				1 1	실						
12:00	1122	77						I d			1			
13:00	1180	42						Ā	PEAK-HOUR VO	DLUME		Yes		No
14:00	1384	79						8	PEAK-HOUR VO	DLUME				
15:00	1679	109		Х		Х					•			
16:00	1674	100		X		Х			WARR PEDESTRIAN V			Yes		No
17:00	1571	41				1 1				OLUME				
18:00	1002	15							WADD	ANTE	1	.,		
19:00	683	15							WARR SCHOOL CROS			Yes		No
20:00	468	11												
21:00	397	6							WADD.	ANT	1	V		NI-
I	_# .								WARR		TEM	Yes		No
Hours M	et : Requirements:	MA IOD.	0 Hours 600	2 Hours 900	0 Hours 480	2 Hours 720								
/olullie	requirements.	MAJOR:	150	100	120	80			WARR	ANT 7	1	Voo		No
		MINOR:	150						ACCIDENT EXP			Yes		No
Davies	Imfammatian			Increased M	linor Street Vo	lı Increased M	inor Street Vol	ume		0040	l l	0040	0040	I
Reviev	v Information	OLIALITY COL	DITC			-				2016	2017	2018	2019	2020
	Counts Used : Count Date(s) :	: QUALITY COU	JN15						R OF ACCIDENTS:	0	3	0	0	2
	Date Reviewed :								ABLE ACCIDENTS:		1	No		0
		: Shrija Ayyars	samv						CTIVE METHODS?  JIREMENTS MET?			No		
	Neviewed by	. Omija 7 tyyare	arriy				ARE	VOLUME REQU	JIREMENTS MET?			110		
									WARR	ANT 8	Ī	Yes		No
									ROADWAY NET					
Comm	ents													
•	01110					-			WARR	ANT 9	Ī	Yes		No
									Intersection Nea	r a Grade Cro	ssing			
								S	STOP OR YIELD (				NORTH	ı
										storage d				
											•	#	%	Adj.
									RAIL	TRAFFIC F	PER DAY =	$\rightarrow$		Factor
								HIGH	H OCCUPANCY E					
								0.7	T ERALL ADJUS	RUCKS PE				<u> </u>
								ÜV	LIVALL ADJUS	INIENI F	AUTUR =			

## STATE OF ILLINOIS KANE COUNTY DISTRICT #1, BUREAU OF TRAFFIC

#### **SUMMARY OF TRAFFIC SURVEY**

INTERSECTION: Kingsland Dr @ E Fabyan Parkway

MUNICIPALITY: Batavia, IL COUNTY: Kane

50075	TRAFFIC FROM NORTH					AFFIC FR						ROM EAS			RAFFIC F				
ROUTE :	_	nd Dr_Com		SRA	_	nd Dr_Co			ı	E Fabya		•	✓ SRA	E Fabyaı		•	✓ SRA		
	N. OF : E Fabyan Parkway				S. OF:	E Fabyar	n Parkwa	ıy		E. OF :				W. OF:					
		GOING		I		GOING		ı	TOTAL		GOING		I		GOING			TOTAL	
	EAST	SOUTH	WEST		WEST	NORTH	EAST		NORTH	SOUTH	WEST	NORTH		NORTH	EAST	SOUTH		EAST	
START					-	<b>↑</b>	_		AND		←	<b>†</b>		<b>↑</b>	<b>→</b>	-1		AND	GRAND
HOUR	<b>—</b>	▼	<b>-</b>	TOTAL		I		TOTAL	SOUTH	▼			TOTAL			•	TOTAL	WEST	TOTAL
6:00	9	2	13	24	7	1	18	26	50	45	334	25	404	17	763	53	833	1237	1287
7:00	6	0	5	11	13	0	18	31	42	43	550	9	602	18	927	60	1005	1607	1649
8:00	9	0	11	20	13	0	19	32	52	28	540	9	577	10	675	29	714	1291	1343
9:00	13	0	12	25	13	0	28	41	66	21	479	10	510	10	507	25	542	1052	1118
10:00	12	0	11	23	13	0	17	30	53	11	459	10	480	11	450	8	469	949	1002
11:00	12	0	9	21	35	0	31	66	87	23	510	13	546	13	481	24	518	1064	1151
12:00	9	0	14	23	36	0	68	104	127	31	551	13	595	8	497	41	546	1141	1268
13:00	12	0	10	22	22	0	34	56	78	25	568	17	610	13	549	31	593	1203	1281
14:00	10	0	12	22	38	0	103	141	163	36	699	11	746	8	619	29	656	1403	1566
15:00	14	0	8	22	42	0	96	138	160	17	1031	18	1066	7	604	18	629	1695	1855
16:00	21	0	20	41	43	0	81	124	165	9	1063	11	1083	19	595	6	620	1703	1868
17:00	14	1	7	22	16	0	56	72	94	15	958	10	983	7	587	11	605	1588	1682
18:00	10	0	8	18	7	0	21	28	46	6	586	10	602	6	405	5	416	1019	1065
19:00	10	0	7	17	10	0	9	19	36	4	386	9	399	7	287	6	300	699	735
20:00	10	0	7	17	7	0	6	13	30	1	276	9	286	6	188	3	197	484	514
21:00	10	0	7	17	4	0	3	7	24	1	224	9	234	7	171	1	179	413	437

Combined Volumes Combined Volumes

#### **REVIEW INFORMATION**

COUNTS USED: QUALITY COUNTS

COUNT DATE(S): 03.29.202 - AM 03.29.202 - PM

DATE REVIEWED: 04.29.2022 REVIEWED BY: Shrija Ayyarsamy Note 1 Minor Leg - High Volume Approach Note 2 20 and 21 North Leg is the minor leg

Note 3 Combined volumes indicate that Settler's & Kingsland's Turning Movements have been combined

#### STATE OF ILLINOIS DEPARTMENT OF TRANSPORTATION BUREAU OF TRAFFIC

DISTRICT #1

#### **RIGHT TURN FACTORIZATION SHEET**

INTERSECTION: Kingsland Drive\_Combined @ E Fabyan Parkway

MUNICIPALITY: Batavia, IL COUNTY: Kane County

DIR	HOUR . BEGIN	L LEFT	MINOR S STREET N CONFIG. #: VOLU T THROUGH	Kingslan 2	d Drive_0	CRITICAL MAINLINE APPROACH VOLUME PER LANE	BASE RIGHT TURN REDUCTION %	MAINLINE CONGESTION FACTOR %	ADJUSTED RIGHT TURN REDUCTION %	ADJUSTED RIGHT TURNS	ADJUSTED MINOR STREET VOLUMES
	6:00	7	1	18	26	382	60%	0%	60%	7	15
	7:00	13	0	18	31	464	60%	5%	55%	8	21
	8:00	13	0	19	32	338	60%	0%	60%	8	21
	9:00	13	0	28	41	254	60%	0%	60%	11	24
	10:00	13	0	17	30	225	60%	0%	60%	7	20
	11:00	35	0	31	66	241	60%	0%	60%	12	47
	12:00	36	0	68	104	249	60%	0%	60%	27	63
	13:00	22	0	34	56	275	60%	0%	60%	14	36
	14:00	38	0	103	141	310	60%	0%	60%	41	79
	15:00	42	0	96	138	302	60%	0%	60%	38	80
	16:00	43	0	81	124	298	60%	0%	60%	32	75
	17:00	16	0	56	72	294	60%	0%	60%	22	38
	18:00	7	0	21	28	203	60%	0%	60%	8	15
	19:00	10	0	9	19	144	60%	0%	60%	4	14
	20:00	10	0	7	17	138	60%	0%	60%	3	13
	21:00	10	0	7	17	112	60%	0%	60%	3	13

Base reduction factor based On Lane configuration 2

#### REVIEW INFORMATION

MAINLINE CONGES	STION FACTORS
VOLUMES	FACTOR (%)
0-399	0
400-499	5
500-599	10
600-699	15
700-799	20
800-899	25
900-999	30
1000-1099	35
1100-1199	40
1200-1299	45
1300-1399	50
1400-1499	55

COUNT DATE(S): 03.29.2022

DATE REVIEWED: 04.29.2022

REVIEWED BY: Shrija Ayyarsamy

COUNTS USED: QUALITY COUNTS

#### **Lane Configurations**











BASE											I===			
REDUCTION	T/4	T/2	3L	3R	(T+R)	(T+L)	T/3	3T	.35A	.7A	TOTAL (A)	RIGHT	THROUGH	LEFT
60%	0	1	21	54	19	8	0	3	9	18	26	18	1	7
60%	0	0	39	54	18	13	0	0	11	22	31	18	0	13
60%	0	0	39	57	19	13	0	0	11	22	32	19	0	13
60%	0	0	39	84	28	13	0	0	14	29	41	28	0	13
60%	0	0	39	51	17	13	0	0	11	21	30	17	0	13
60%	0	0	105	93	31	35	0	0	23	46	66	31	0	35
60%	0	0	108	204	68	36	0	0	36	73	104	68	0	36
60%	0	0	66	102	34	22	0	0	20	39	56	34	0	22
60%	0	0	114	309	103	38	0	0	49	99	141	103	0	38
60%	0	0	126	288	96	42	0	0	48	97	138	96	0	42
60%	0	0	129	243	81	43	0	0	43	87	124	81	0	43
60%	0	0	48	168	56	16	0	0	25	50	72	56	0	16
60%	0	0	21	63	21	7	0	0	10	20	28	21	0	7
60%	0	0	30	27	9	10	0	0	7	13	19	9	0	10
60%	0	0	30	21	7	10	0	0	6	12	17	7	0	10
60%	0	0	30	21	7	10	0	0	6	12	17	7	0	10

#### NOT On

the minor street, the higher volume shall not be required to be on the same approach during each of these 8 hours.

### **SIGNAL WARRANT REVIEW SHEET**

ILLINOIS DEPARTMENT OF TRANSPORTATION

SRA : **E Fabyan Pakrway**Yes No

	Intersection	: Kingsland D	rive @ E	Fabyan F	Parkway			County:	Kane County			
	Municipality	: Batavia, IL			_							
	Speed Lir Number of Lanes o	mit of Major Route on Major approach	45 mph 2	- -	Isola			ulation < 10,000 Minor approach	<u>N</u> 2			
			CHE	CK ANY HOURS W	HICH MEET THE	FOLLOWING WAI	RRANTS		WARRANT 1	Yes		No
	Major Street Volume (both	Adj. Minor Street Volume	-	RANT 1	11	T 7: 8 hrs of one			Warrant 1 is met if any of the fo		tions are	
	approaches)	(higher volume	A	В					Condition A	lowing Condi	Yes	No
HOUR BEGIN		approach)	100%	100%	80% of A	80% of B	80% of Warr		MINIMUM VEHICULAR VOLUME		100	NO
6:00	1237	15	10070	10070	0070017	00 % OI B	00 % OI VVaii	_	Condition B		Yes	No
7:00	1607	21							INTERRUPTION OF CONTINUOUS	TRAFFIC	163	NO
8:00	1291	21						∃ ∢⊏	•	0 hours	Yes	No
9:00	1052	24						(SRA)			103	140
10:00	949	20						_ " _ [		Yes		No
11:00	1064	47		11	11	H		AB		165		INO
12:00	1141	63			1							
13:00	1203	36						AP		Yes		No
14:00	1403	79						NOT APPLICABLE	PEAK-HOUR VOLUME	100		140
15:00	1695	80										
16:00	1703	75							<b>WARRANT 4</b>	Yes		No
17:00	1588	38		11					PEDESTRIAN VOLUME			
18:00	1019	15										
19:00	699	14							<b>WARRANT 5</b>	Yes		No
20:00	484	13							SCHOOL CROSSING			
21:00	413	13										
					•			_	<b>WARRANT 6</b>	Yes		No
Hours M	et:		0 Hours	0 Hours	0 Hours	1 Hours			COORDINATED SIGNAL SYSTEM			
/olume	Requirements:	MAJOR:	600	900	480	720		_				
		MINOR:	200	150	160	120		_ [	<b>WARRANT 7</b>	Yes		No
				Increased V	olume	Increased \	_ √olume	ı	ACCIDENT EXPERIENCE			
Reviev	w Information								YEAR: 2016 201	7 2018	2019	2020
	Counts Used	: QUALITY CO	UNTS			_		TOTAL NUMBER O	F ACCIDENTS: 3 6	3	1	2
	Count Date(s)	: 03.29.2022					1	NUMBER CORRECTABL	E ACCIDENTS: 0 1	0	0	0
	Date Reviewed						т	RIED LESS RESTRICTION	VE METHODS?	No		
	Reviewed By	: Shrija Ayyars	samy					ARE VOLUME REQUIRE	EMENTS MET? 3S Yes (Acci	dent Vol	Requi	iremen
								ı				
									WARRANT 8	Yes		No
									NOADWAT NETWORK			
Comm	ents					_		ı	WARRANT O			
									WARRANT 9 Intersection Near a Grade Crossing	Yes		No
								STO	P OR YIELD CONTROLLED LEG			
								0.0	WITH GRADE CROSSING:		NORTH	l .
									D (clear storage distance	) =		1 A 41:
										#	%	Adj. Factor
								нісь с	RAIL TRAFFIC PER DA'			-
								HIGH	TRUCKS PER HOU			+
								OVER	RALL ADJUSTMENT FACTOR	≀ =		
											·	· <u></u>



**ATTACHMENT 4: HCS Reports** 



#### **HCS Results Summary**

#### • Kingsland (Existing Geometry) - seconds (approach delay)

Peak Period	NB TWSC	SB TWSC
AM Existing	43	70
AM 2050	376	337
PM Existing	34	50
PM 2050	383	249

#### Kingsland (Alternative "B") - seconds (approach delay)

Peak Period	NB TWSC	SB TWSC
AM Existing	43	78
AM 2050	197	280
PM Existing	26	51
PM 2050	113	207

#### • Settler's Hill (Existing Conditions) - seconds (approach delay)

Peak Period	SB Signalized
AM Existing	62
AM 2050	62
PM Existing	58
PM 2050	58

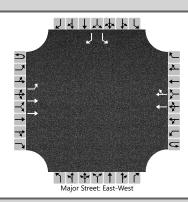
#### • <u>Settler's Hill (Alternative "B") - seconds (approach delay)</u>

Peak Period	SB TWSC
AM Existing	15
AM 2050	21
PM Existing	36
PM 2050	108

### • <u>Combined intersection Worst Delays under proposed conditions (Overall Intersection and EW approaches are B or better) - seconds (approach delay)</u>

Peak Period	NB TWSC	NB Signalized	SB TWSC	SB Signalized
AM Existing	47	82	47	69
AM 2050	237	94	180	68
PM Existing	27	61	79	51
PM 2050	135	60	526	50

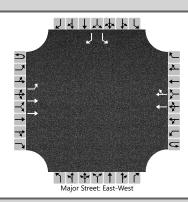
	HCS7 Two-Way Stop-Control Report												
General Information		Site Information											
Analyst	Shrija Ayyarsamy	Intersection	Batavia, Illinois										
Agency/Co.	Peralte-Clark, LLC	Jurisdiction	Kane County										
Date Performed	6/2/2022	East/West Street	E Fabyan Parkway										
Analysis Year	2022	North/South Street	Settler's Hill Driveway										
Time Analyzed	AM 2022_Proposed Geometry	Peak Hour Factor	0.87										
Intersection Orientation	East-West	Analysis Time Period (hrs) 0.25											
Project Description	Settler's and Kingsland Feasibility Study												



Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		1	0	1
Configuration		L	Т				Т	TR						L		R
Volume (veh/h)	0	17	986				561	9						2		5
Percent Heavy Vehicles (%)	0	5												0		0
Proportion Time Blocked																
Percent Grade (%)	1													(	0	
Right Turn Channelized													No			
Median Type   Storage	rage Undivided															
<b>Critical and Follow-up He</b>																
Base Critical Headway (sec)		4.1												7.5		6.9
Critical Headway (sec)		4.20												6.80		6.90
Base Follow-Up Headway (sec)		2.2												3.5		3.3
Follow-Up Headway (sec)		2.25												3.50		3.30
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1	20												2		6
Capacity, c (veh/h)		908												163		674
v/c Ratio		0.02												0.01		0.01
95% Queue Length, Q <sub>95</sub> (veh)		0.1												0.0		0.0
Control Delay (s/veh)		9.1												27.5		10.4
Level of Service (LOS)		А												D		В
Approach Delay (s/veh)	0.2												15.3			
Approach LOS													С			

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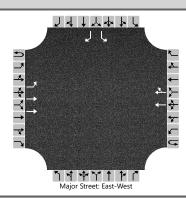
	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Shrija Ayyarsamy	Intersection	Batavia, Illinois
Agency/Co.	Peralte-Clark, LLC	Jurisdiction	Kane County
Date Performed	6/2/2022	East/West Street	E Fabyan Parkway
Analysis Year	2050	North/South Street	Settler's Hill Driveway
Time Analyzed	AM 2050_Proposed Geometry	Peak Hour Factor	0.87
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Settler's and Kingsland Feasibility Study		



Vehicle Volumes and Adju	ustme	nts														
Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		1	0	1
Configuration	1	L	Т				Т	TR						L		R
Volume (veh/h)	0	23	1341				764	12						2		6
Percent Heavy Vehicles (%)	0	5												0		0
Proportion Time Blocked	1															
Percent Grade (%)	1													(	)	
Right Turn Channelized	1													Ν	lo	
Median Type   Storage	1			Undi	vided											
Critical and Follow-up He	eadwa	ys														
Base Critical Headway (sec)	1	4.1												7.5		6.9
Critical Headway (sec)		4.20												6.80		6.90
Base Follow-Up Headway (sec)	1	2.2												3.5		3.3
Follow-Up Headway (sec)	1	2.25												3.50		3.30
Delay, Queue Length, and	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1	26												2		7
Capacity, c (veh/h)	1	737												81		565
v/c Ratio	1	0.04												0.03		0.01
95% Queue Length, Q <sub>95</sub> (veh)		0.1												0.1		0.0
Control Delay (s/veh)		10.1												50.9		11.4
Level of Service (LOS)		В												F		В
Approach Delay (s/veh)		0	.2										21.3			
Approach LOS	1												С			

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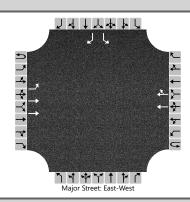
	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Shrija Ayyarsamy	Intersection	Batavia, Illinois
Agency/Co.	Peralte-Clark, LLC	Jurisdiction	Kane County
Date Performed	6/2/2022	East/West Street	E Fabyan Parkway
Analysis Year	2022	North/South Street	Settler's Hill Driveway
Time Analyzed	PM 2022_Proposed Geometry	Peak Hour Factor	0.98
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Settler's and Kingsland Feasibility Study		



Vehicle Volumes and Adju	ıstme	nts															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		1	0	1	
Configuration		L	Т				T	TR						L		R	
Volume (veh/h)	0	11	630				1123	18						19		6	
Percent Heavy Vehicles (%)	0	6												0		0	
Proportion Time Blocked																	
Percent Grade (%)														(	0		
Right Turn Channelized	1													Ν	lo		
Median Type   Storage	1			Undi	vided												
Critical and Follow-up He	adwa	ys															
Base Critical Headway (sec)		4.1												7.5		6.9	
Critical Headway (sec)		4.22												6.80		6.90	
Base Follow-Up Headway (sec)		2.2												3.5		3.3	
Follow-Up Headway (sec)		2.26												3.50		3.30	
Delay, Queue Length, and	l Leve	l of Se	ervice														
Flow Rate, v (veh/h)	1	11												19		6	
Capacity, c (veh/h)		572												113		461	
v/c Ratio		0.02												0.17		0.01	
95% Queue Length, Q <sub>95</sub> (veh)		0.1												0.6		0.0	
Control Delay (s/veh)	1	11.4												43.4		12.9	
Level of Service (LOS)	1	В												E		В	
Approach Delay (s/veh)		0	.2											36.1			
Approach LOS	1												E				

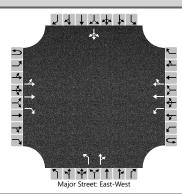
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	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Shrija Ayyarsamy	Intersection	Batavia, Illinois
Agency/Co.	Peralte-Clark, LLC	Jurisdiction	Kane County
Date Performed	6/2/2022	East/West Street	E Fabyan Parkway
Analysis Year	2050	North/South Street	Settler's Hill Driveway
Time Analyzed	PM 2050_Proposed Geometry	Peak Hour Factor	0.98
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Settler's and Kingsland Feasibility Study		



Vehicle Volumes and Adj	ustille															
Approach	<u> </u>	Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	10	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		1	0	1
Configuration	1	L	Т				Т	TR						L		R
Volume (veh/h)	0	15	857				1529	24						23		7
Percent Heavy Vehicles (%)	0	6												0		0
Proportion Time Blocked	1															
Percent Grade (%)	1													(	0	
Right Turn Channelized	1													Ν	lo	
Median Type   Storage	1			Undi	vided											
Critical and Follow-up Ho	eadwa	ys														
Base Critical Headway (sec)	1	4.1												7.5		6.9
Critical Headway (sec)		4.22												6.80		6.90
Base Follow-Up Headway (sec)		2.2												3.5		3.3
Follow-Up Headway (sec)		2.26												3.50		3.30
Delay, Queue Length, and	d Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1	15												23		7
Capacity, c (veh/h)	1	391												48		336
v/c Ratio		0.04												0.49		0.02
95% Queue Length, Q <sub>95</sub> (veh)		0.1												1.8		0.1
Control Delay (s/veh)		14.6												136.6		15.9
Level of Service (LOS)	1	В												F		С
Approach Delay (s/veh)	0.3										-		108.5			
Approach LOS													F			

	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Shrija Ayyarsamy	Intersection	Batavia, Illinois
Agency/Co.	Peralte-Clark, LLC	Jurisdiction	Kane County
Date Performed	4/28/2022	East/West Street	E Fabyan Parkway
Analysis Year	2022	North/South Street	Kingsland Drive
Time Analyzed	AM 2022_Proposed Geometry	Peak Hour Factor	0.89
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Settler's and Kingsland Feasibility Study		

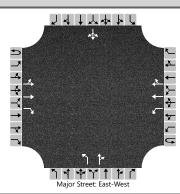


Vehicle Volumes and Adjustments																
Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			Westk	ound			North	bound			South	bound	
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	2	1	0	1	2	0		1	1	0		0	1	0
Configuration		LT	Т	R		L	Т	TR		L		TR			LTR	
Volume (veh/h)		1	927	60	0	43	550	0		13	0	18		3	0	0
Percent Heavy Vehicles (%)		0			3	28				38	0	67		100	0	0
Proportion Time Blocked		0.000				0.000				0.000	0.000	0.000		0.000	0.000	0.000
Percent Grade (%)										(	0				0	
Right Turn Channelized		N	lo													
Median Type   Storage	1	Undivided														
<b>Critical and Follow-up He</b>	adwa	lways														
Base Critical Headway (sec)	1	4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)		4.10				4.66				8.26	6.50	8.24		9.50	6.50	6.90
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20			2.48					3.88	4.00	3.97	4.50 4.00 3.30			
Delay, Queue Length, and	Leve	l of Se	ervice													
Flow Rate, v (veh/h)	1	1				48				15		20			3	
Capacity, c (veh/h)		972				494				61		362			52	
v/c Ratio		0.00				0.10				0.24		0.06			0.06	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.3				0.8		0.2			0.2	
Control Delay (s/veh)	1	8.7				13.1				81.9		15.5			78.5	
Level of Service (LOS)		А			В			F C						F		
Approach Delay (s/veh)	1	0	.0		0.9				43.4				78.5			
Approach LOS	1										E		F			

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#### HCS7 Two-Way Stop-Control Report Site Information **General Information** Analyst Shrija Ayyarsamy Intersection Batavia, Illinois Agency/Co. Peralte-Clark, LLC Jurisdiction Kane County Date Performed 4/28/2022 East/West Street E Fabyan Parkway 2050 Kingsland Drive Analysis Year North/South Street Time Analyzed AM 2050\_Proposed Geometry Peak Hour Factor 0.89 Analysis Time Period (hrs) Intersection Orientation East-West 0.25 **Project Description** Settler's and Kingsland Feasibility Study

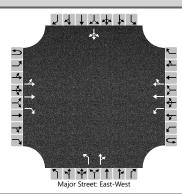
#### Lanes



Vehicle Volumes and Adju	ıstme	nts															
Approach		Eastb	ound			Westk	ound			North	bound			South	bound		
Movement	U	L	Т	R	U	L	T	R	U	L	T	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	2	1	0	1	2	0		1	1	0		0	1	0	
Configuration		LT	Т	R		L	Т	TR		L		TR			LTR		
Volume (veh/h)		1	1258	81	0	59	751	0		17	0	23		4	0	0	
Percent Heavy Vehicles (%)		0			3	28				38	0	67		100	0	0	
Proportion Time Blocked		0.000				0.000				0.000	0.000	0.000		0.000	0.000	0.000	
Percent Grade (%)										(	0				0		
Right Turn Channelized		N	lo														
Median Type   Storage	1	Undivided															
<b>Critical and Follow-up He</b>	adwa	dways															
Base Critical Headway (sec)	1	4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9	
Critical Headway (sec)		4.10				4.66				8.26	6.50	8.24		9.50	6.50	6.90	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)	1	2.20			2.48					3.88	4.00	3.97	4.50 4.00 3.30				
Delay, Queue Length, and	Leve	l of Se	ervice														
Flow Rate, v (veh/h)	1	1				66				19		26			4		
Capacity, c (veh/h)		801				333				21		259			17		
v/c Ratio		0.00				0.20				0.93		0.10			0.26		
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.7				2.6		0.3			0.7		
Control Delay (s/veh)	1	9.5				18.5				436.4		20.4			279.8		
Level of Service (LOS)		А			С			F C						F			
Approach Delay (s/veh)	1	0	.0		1.3				197.2				279.8				
Approach LOS											F		F				

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	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	Shrija Ayyarsamy	Intersection	Batavia, Illinois
Agency/Co.	Peralte-Clark, LLC	Jurisdiction	Kane County
Date Performed	4/28/2022	East/West Street	E Fabyan Parkway
Analysis Year	2022	North/South Street	Kingsland Drive
Time Analyzed	PM 2022_Proposed Geometry	Peak Hour Factor	0.92
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Settler's and Kingsland Feasibility Study		

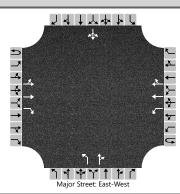


	umes and Adiustments															
Vehicle Volumes and Adju	ıstme	nts														
Approach		Eastb	ound			West	oound			North	bound			South	bound	
Movement	U	L	T	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	2	1	0	1	2	0		1	1	0		0	1	0
Configuration		LT	Т	R		L	Т	TR		L		TR			LTR	
Volume (veh/h)		0	619	14	0	13	1060	8		46	0	97		9	0	4
Percent Heavy Vehicles (%)		0			3	31				2	0	10		0	0	0
Proportion Time Blocked						0.000				0.000	0.000	0.000		0.000	0.000	0.000
Percent Grade (%)										(	0				0	
Right Turn Channelized		Ν	lo													
Median Type   Storage		Undivided														
<b>Critical and Follow-up He</b>	adwa	dways														
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9
Critical Headway (sec)		4.10				4.72				7.54	6.50	7.10		7.50	6.50	6.90
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3
Follow-Up Headway (sec)		2.20			2.51					3.52	4.00	3.40	3.50 4.00 3.30			
Delay, Queue Length, and	l Leve	l of Se	ervice													
Flow Rate, v (veh/h)		0				14				50		105			14	
Capacity, c (veh/h)		609				733				120		637			92	
v/c Ratio		0.00				0.02				0.42		0.17			0.15	
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.1				1.8		0.6			0.5	
Control Delay (s/veh)		10.9				10.0				54.8		11.8			50.9	
Level of Service (LOS)		В			В			F B						F		
Approach Delay (s/veh)		0	.0		0.1				25.6				50.9			
Approach LOS										1	)		F			

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#### HCS7 Two-Way Stop-Control Report **General Information** Site Information Analyst Shrija Ayyarsamy Intersection Batavia, Illinois Agency/Co. Peralte-Clark, LLC Jurisdiction Kane County Date Performed 4/28/2022 East/West Street E Fabyan Parkway 2050 Kingsland Drive Analysis Year North/South Street Time Analyzed PM 2050\_Proposed Geometry Peak Hour Factor 0.92 Analysis Time Period (hrs) Intersection Orientation East-West 0.25 **Project Description** Settler's and Kingsland Feasibility Study

#### Lanes



/ehicle Volumes and Adjustments																	
Vehicle Volumes and Adju	ıstme	nts															
Approach		Eastb	ound			Westl	oound			North	bound			South	bound		
Movement	U	L	T	R	U	L	Т	R	U	L	T	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	0	2	1	0	1	2	0		1	1	0		0	1	0	
Configuration		LT	Т	R		L	Т	TR		L		TR			LTR		
Volume (veh/h)		0	840	19	0	18	1447	11		60	0	127		11	0	5	
Percent Heavy Vehicles (%)		0			3	31				2	0	10		0	0	0	
Proportion Time Blocked						0.000				0.000	0.000	0.000		0.000	0.000	0.000	
Percent Grade (%)										(	0				0		
Right Turn Channelized		Ν	lo														
Median Type   Storage		Undivided															
<b>Critical and Follow-up He</b>	adwa	dways															
Base Critical Headway (sec)		4.1				4.1				7.5	6.5	6.9		7.5	6.5	6.9	
Critical Headway (sec)		4.10				4.72				7.54	6.50	7.10		7.50	6.50	6.90	
Base Follow-Up Headway (sec)		2.2				2.2				3.5	4.0	3.3		3.5	4.0	3.3	
Follow-Up Headway (sec)		2.20			2.51					3.52	4.00	3.40	3.50 4.00 3.30				
Delay, Queue Length, and	l Leve	l of Se	ervice														
Flow Rate, v (veh/h)		0				20				65		138			17		
Capacity, c (veh/h)		420				574				53		530			32		
v/c Ratio		0.00				0.03				1.22		0.26			0.54		
95% Queue Length, Q <sub>95</sub> (veh)		0.0				0.1				5.7		1.0			1.8		
Control Delay (s/veh)		13.6				11.5				321.9		14.2			207.2		
Level of Service (LOS)		В			В			F B						F			
Approach Delay (s/veh)		0	.0		0.1				112.9				207.2				
Approach LOS											F		F				



**ATTACHMENT 5: Accident Analysis** 



#### **Accident Analysis**

A total of 15 crashes were identified as occurring along the Fabyan Parkway corridor at/between its intersections with Settler's Hill Golf Course entrance and Kingsland Drive. The study limits were between 2016 and 2020.

- 15 crashes with minor injuries or less
- 8 rear ends
- 3 sideswipe
- Potential Cause: Lack of left turning lanes

### **CRASH ANALYSIS REPORT**

### Settler's Hill and Kingsland Drive Feasibility Study

Fabyan Parkway, Settler's Hill Golf Course Entrance to Kingsland Drive

Prepared For:



41W011 Burlington Rd, Campton Hills, IL 60175

Prepared By:



44 S Vail Ave, Suite #201 Arlington Heights, IL 60005

June 2022



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Introduction	2
Crash Analysis Summary	3
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Fabyan Parkway at Kingsland Drive Intersection	8
Data Summary	8
Data Analysis	9
Potential Actions	

### **LIST OF ATTACHMENTS**

- A. KABC Crash Data Tables
  - Settler's Hill Golf Course Entrance
  - Kingsland Drive Intersection



#### Introduction

The Kane County Division of Transportation (KDOT) has initiated a feasibility study of improvement needs on Fabyan Parkway at the intersections with the Settlers Hill Golf Course Entrance and Kingsland Drive. A key element of the feasibility study is to evaluate crash history and assess the overall safety of the intersections under study. The study limits extend from west of Settler's Hill Drive to east of Kingsland Drive. The purpose of this crash analysis report is to evaluate existing crash data, identify patterns and potential causes, and to provide recommendations for roadway improvements which might lead to a reduction in crash frequency and/or severity.

See figure below, for study limits:



Evaluating historical data is a three-step process: 1) Data Collection, 2) Data Processing, and 3) Data Analysis. The process is described below, and the results of the analysis are included in the subsequent sections of this report.

#### **Data Collection**

Crash reports from the state and local police were obtained by the Kane County Division of Transportation. These included data for the years 2016 through 2020.

#### **Data Processing**

The crash data is summarized by various categories: location of crash (intersection or segment), crash type, and severity of crashes. Crashes are also analyzed by roadway surface condition, weather condition, roadway lighting condition, and by date and time the crashes occurred. This provides an indication if the roadway surface, weather, lighting conditions and/or time and day of occurrence are contributing factors to the crashes being experienced.

#### **Data Analysis**

Each intersection was evaluated by analyzing crash patterns (number, type, and severity). A high concentration of certain crash types at a particular location is typically an indication of a common cause or reason. Crash severity, type and location are key indicators in the evaluation of current safety conditions of the corridor. Analysis of the data was performed to determine probable causes for the crashes recorded at the intersections. The geometric layout of the roadway, traffic conditions, and other site conditions were used to develop the probable causes of crashes and ultimately assist in development of potential safety improvements.



#### **Injury Severity Descriptions**

Crashes are categorized at five severity levels: Type K (Fatal), Type A (Incapacitating), Type B (Non-Incapacitating), Type C (Reported, Injury not Evident), and Property Damage Only (PDO). A brief description of each of the crash severity types is provided below:

- Type K (Fatal) A crash that involves at least one person who dies within 30 days of the crash.
- Type A (Incapacitating Injury) A crash that involves any injury, other than fatal, that prevents the injured person from walking, driving, or normally continuing the activities he/she was capable of performing before the injury occurred. Inclusions: severe lacerations, broken/distorted limbs, skull injuries, chest injuries and abdominal injuries.
- Type B (Non-Incapacitating Injury) A crash that involves any injury, other than a fatal or incapacitating injury, that is evident to observers at the scene of the crash. Inclusions: lumps on the head, abrasions, bruises, and minor lacerations.
- Type C (Reported, Injury not Evident) A crash that involves any injury reported or claimed that is not listed
  above and that is not evident to observers at the scene of the crash. Inclusions: momentary unconsciousness,
  claims of injuries not evident, limping, and complains of pain or nausea.
- **Property Damage Only (PDO)** A crash that involves no injuries or fatalities, but damage is caused to either vehicle and the costs of the damage is assessed to be greater than \$1500.

### **Crash Analysis Summary**

A total of 15 crashes were identified as occurring along the Fabyan Parkway corridor at/between its intersections with Settler's Hill Golf Course Entrance and Kingsland Drive.

#### Crash Frequency

A total of 15 crashes occurred within the study limits between 2016 and 2020. A review of the frequency of crashes by year indicates a slight increase in the year 2017 over other years (2016 and 2018-2020) in the data set. Specific cause of this increase for the peak in 2017 is unknown. Of the total 15 crashes, 6 (40.0%) crashes were reported as injury crashes, resulting in injuries of varying degrees of severity. There were no fatalities recorded during the study period. The following table summarizes the five-year data set by year for total crashes and injury crashes.



	Total Crash Data, Years (2016 – 2020)														
Year	Tota	l Crashes	Inju	ry Crashes	Number of Injuries by Type										
	Total	Frequency	Total	Frequency	K	Α	В	С	Total						
2016	3	20.0%	1	16.7%				1	1						
2017	6	40.0%	4	66.6%			3	1	4						
2018	3	20.0%	1	16.7%				1	1						
2019	1	6.7%	0	0.0%											
2020	2	13.3%	0	0.0%											
Total	15	100.0%	6	N/A	0	0	3	3	6						

#### **Crash Type Distribution**

The following table describes the distribution of the total 15 crashes within the study limits by crash type. The most common crash types were rear end (53.3%) and sideswipe same direction (20.0%). Probable causes of these types of crashes include deceleration of turning vehicles, over speeding, and lane changing. Other crash types in the data set include turning (13.3%) and fixed object and animal crashes (6.7% each).

	Total Crash Data, Distributed by Type													
Crash Type	Total (	Crashes	Number of Injuries by Crash Type											
Crash Type	Total	К	Α	В	С	Total	Frequency							
Animal	1	6.7%						0.0%						
Fixed Object	1	6.7%						0.0%						
Turning	2	13.3%			2		2	13.3%						
Rear End	8	53.3%			1	3	4	66.7%						
Sideswipe Same Direction	3	20.0%						0.0%						
Total	15	100.0%	0	0	3	3	6	100.0%						



#### **Crash Severity**

Six (6) crashes (40.0%) involved injuries. These injury crashes resulted in a total of 6 injuries of varying severity. There were no fatalities (Type K) or Incapacitating injury (Type A) crashes during the study period. The following table shows the total number of injuries recorded by crash type.

Number of Injuri	es by Cras	sh Type
Crash Type		r of Injuries / Type
	Total	Frequency
Turning	2	13.3%
Rear End	4	66.7%
Total	6	100.0%

Of the 6 injuries recorded, 4 (66.7%) were the result of rear end collisions and 2 (13.3%) were the result of turning collisions. The severity of crashes was distributed as: 3 (50.0%) Type B, and 3 (50.0%) Type C injuries. The remaining crash reports were identified as either no damage or property damage only and not included as part of the injury severity analysis.

#### **Roadway Surface Condition Analysis**

The table to the right summarizes the distribution of crashes by roadway surface condition. As shown, 86.7% of all crashes occurred on a dry roadway surface, with 13.3% of crashes occurring on wet, snowy, slushy, or icy pavement conditions. The general findings of this crash analysis suggest that wet pavement was not a significant cause of crashes within the study area. These percentages do not indicate any particular deficiency in drainage or skid resistance.

Total Crashes, Distribu	ted by Roadw	ay Condition								
Roadway Condition	Total Crashes									
,	Total	Frequency								
Wet Pavement	2	13.3%								
Snow/Slush/Ice	1	0.0%								
Dry Pavement	13	86.7%								
Unknown	-1	0.0%								
Totals	15	100.0%								



#### Roadway Lighting Condition Analysis

The table to the right summarizes the distribution of crashes by lighting condition. Over the study period, over 90% of all crashes occurred in daylight or during dawn/dusk, which indicates that lack of street lighting did not contribute to a significant occurrence of crashes.

Total Crashes, Distribute	Total Crashes, Distributed by Lighting Condition											
Lighting Condition	Total Crashes											
	Total	Frequency										
Darkness	1	6.7%										
Darkness, Lighted Road		0.0%										
Dawn/Dusk	8	53.3%										
Daylight	6	40.0%										
Unknown		0.0%										
Totals	15	100.0%										

#### Time of Day Analysis

The table to the right summarizes the distribution of crashes by the time of day at the time of the crash. Morning hours (4:00 - 7:59 am) resulted in the highest distribution of crashes, followed by the Noon - 3:59 pm time frame. It is noted that only one (1) crash occurred between 8:00 pm and 4:00 am, which indicates that evening hours are not a significant concern. The higher distribution in the morning hours is indicative of morning rush hour / peak hour traffic expected.

Total Crashes, Distrib	uted by Time	of Day
Lighting Condition	Total (	Crashes
	Total	Frequency
Midnight - 3:59 am	1	6.7%
4:00 - 7:59 am	5	33.3%
8:00 - 11:59 am	2	13.3%
Noon - 3:59 pm	4	26.7%
4:00 - 7:59 pm	3	20.0%
8:00 - 11:59 pm		0.0%
Totals	15	100.0%



#### Settler's Hill Golf Course Entrance



The intersection of Fabyan Parkway at Settler's Hill Golf Course has a temporary traffic signal. At this T intersection, the north leg includes an unmarked 25-foot lane which functions as exclusive left and right turn lanes. The east and west legs are comprised of two through lanes. There are no excusive left and right turn lanes. The lanes are 12 feet wide.

There are no pedestrian accommodations at this intersection. There is a multi-use bike path located at the northern quadrants of the intersection.

Street lighting is limited to the intersection footprint with no approach lighting. The adjacent land use is primarily recreational (golf course), industrial and commercial.

#### **Data Summary**

A total of five (5) crashes occurred at the Fabyan Parkway and Settler's Hill Golf Course entrance during the study period, representing 33.3% of the total 15 crashes within the study limits. Of the 5 crashes, 3 (60.0%) were categorized as rear end crashes and 1 (20.0% each) as turning crashes and fixed object crashes. Three (3) injuries were reported at this intersection and were distributed as two Type B, and one Type C.

All five of the recorded crashes occurred on dry pavement. A total of 2 crashes were reported as having occurred under darkness and dawn/dusk, while the remaining 3 crashes occurred during daylight.

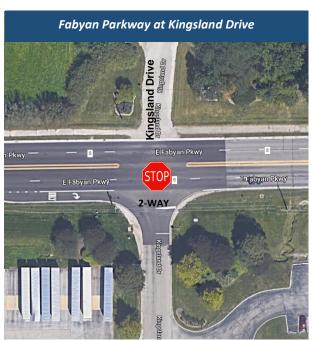
Crash Data Summ	Crash Data Summary, Fabyan Parkway at Settler's Hill Golf Course Entrance														
Crash Type	Tota	l Crashes	Number of Injuries by Type												
3.33, <b>7</b> p2	Total	Frequency	К	Α	В	С	Total	Frequency							
Fixed Object	1	20.0%						0.0%							
Turning	1	20.0%			1		1	33.3%							
Rear End	3	60.0%			1	1	2	66.7%							
Total	5	100.0%	0	0	2	1	3	100.0%							



#### **Data Analysis**

At 60%, rear end was the most common type of crash recorded at this intersection. The fixed object crash occurred when a yellow utility pole was hit. The turning crash occurred when a vehicle waiting to turn westbound left collided with an eastbound vehicle on red. Rear end crashes occurred due to a failure to reduce speed to avoid crashes and distracted driving. There does not appear to be a correlation between pavement condition and the occurrence of any one type of crash as all crashes occurred on dry pavement. The same is true of roadway lighting conditions where 60.0% of crashes occurred during clear, daylight hours.

### **Fabyan Parkway at Kingsland Drive Intersection**



The intersection of Fabyan Parkway at Kingsland Drive is a two-way stop-controlled intersection. The north and south approaches, which are stop-controlled, are one lane each with, shared left, through and right turns. The west approach has two through lanes and an exclusive right tun lane. The east approach has two through lanes.

There are no pedestrian accommodations at this intersection. Street lighting is limited to a single lamp on an existing utility pole in the southwest quadrant. The adjacent land use is industrial and commercial with access points at the southeast, and southwest quadrants of this intersection.

#### **Data Summary**

A total of ten (10) crashes occurred at this intersection during the study period, representing 66.6% of the total 15 crashes within the study limits. Of the 10 crashes, 5 (50.0%) were categorized as rear

end crashes, 3 (30.0%) were sideswipe same direction crashes, the remaining 2 (10.0%) were turning and animal crashes each. Three injuries were reported at this intersection, two (2) were categorized as Type C injuries and the remaining crash was classified as Type B.

Of the 10 crashes, 2 crashes were reported as having occurred when the pavement was wet. Three crashes occurred during the day, while the remaining seven (7) crashes occurred during dawn/dusk. No crash occurred during night.



Crash Data Sur	Crash Data Summary, Fabyan Parkway at Kingsland Drive Intersection														
Crash Type	Tota	al Crashes	Number of Injuries by Type												
Crash Type	Total	Frequency	К	Α	В	С	Total	Frequency							
Animal	1	10.0%					0	0.0%							
Turning	1	10.0%			1		1	33.3%							
Read End	5	50.0%				2	2	66.7%							
Sideswipe Same Direction	3	30.0%					0	0.0.%							
Total	10	100.0%	0	0	1	2	3	100.0%							

#### Data Analysis

At a combined 80.0%, rear-end and sideswipe same direction were the most common type of crashes recorded at this intersection. The remaining crash types recorded at this intersection (turning and animal) were determined to be negligible. Application of sudden brakes and failure to reduce speed were the primary reasons for the high incidence of rear-end crashes. Review of the crash information also shows that frequent lane shifts to avoid turning vehicles contributes to the sideswipe same direction crashes.

Seven of ten crashes occurred during low light conditions (dawn/dusk) which indicates a lack of visibility. Eight of ten crashes occurred on dry pavement, which indicates weather conditions did not contribute significantly to crashes recorded.

#### **Potential Actions**

Based on trends observed within the study limits, the following potential actions are recommended:

- Consider exclusive left turn lanes
- Consider roadway lighting or traffic signal adjustments to improve visibility, such as LED lamps or reflective backplates
- Consider consolidating into one access point

Project Name: Settler's and Kingsland Feasibility Study
Crash Data from (2016 TO 2020)
Crash Data Location: Fabyan Parkway @ Settler's Hill Golf Course Entrance

			2016					2017					2018					2019					2020					Totals			
Crash Type	# Crashes	# K fat.	# A inj.	# B inj.	# C inj.	# Crashes	# K fat.	# A inj.	# B inj.	# C inj.	# Crashes	# K fat.	# A inj.	# B inj.	# C inj.	# Crashes	# K fat.	# A inj	j. # B inj.	# C inj.	# Crashes	# K fat.	# A inj.	# B inj.	# C inj.	# Crashes	% Crashes	# K fat.	# A inj.	# B inj.	# C inj.
Pedestrian																										0	0.0%	0	0	0	0
Pedalcyclist																										0	0.0%	0	0	0	0
Train																										0	0.0%	0	0	0	0
Animal																										0	0.0%	0	0	0	0
Overturned																										0	0.0%	0	0	0	0
Fixed Object						1																				1	20.0%	0	0	0	0
Other Object																										0	0.0%	0	0	0	0
Other Noncollision																										0	0.0%	0	0	0	0
Parked Motor Vehicle																										0	0.0%	0	0	0	0
Turning						1			1																	1	20.0%	0	0	1	0
Rear End	2				1	1			1																	3	60.0%	0	0	1	1
Sideswipe Same Direction																										0	0.0%	0	0	0	0
Sideswipe Opposite Direction																										0	0.0%	0	0	0	0
Head On																										0	0.0%	0	0	0	0
Angle																										0	0.0%	0	0	0	0
Other																										0	0.0%	0	0	0	0
Total	2	0	0	0	1	3	0	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	5	100.0%	0	0	2	1

# Crashes w/Injuries	2016	2017	2018	2019	2020	Total
# Crashes with K fatalities						0
# Crashes with A injuries						0
# Crashes with B injuries		2				2
# Crashes with C injuries	1					1
Totals	1	2	0	0	0	3

Note: Crashes documenting multiple types of injuries are counted only once and in the more serious injury category.

Time of Crash	2016	2017	2018	2019	2020	Total
Darkness		1				1
Darkness, Lighted Road						0
Dawn/Dusk		1				1
Daylight	2	1				3
Unknown						0
Totals	2	3	0	0	0	5

Roadway Condition	2016	2017	2018	2019	2020	Total
# Crashes with wet pavement						0
# Crashes with snow/slush/ice						0
# Crashes with dry pavement	2	3				5
# Unknown Condition						0
Totals	2	3	0	0	0	5

Crash Data from (2016 TO 2020)
Project Name: Settler's and Kingsland Feasibility Study
Crash Data Location: Fabyan Parkway @ Settler's Hill Golf Course Entrance
Kane County

Time of Day	2016	2017	2018	2019	2020	Total
Midnight - 3:59 am		1				1
4:00 - 7:59 am						0
8:00 - 11:59 am	2					2
Noon - 3:59 pm		1				1
4:00 - 7:59 pm		1				1
8:00 - 11:59 pm						0
Totals	2	3	0	0	0	5

# Project Name: Settler's and Kingsland Feasibility Study Crash Data from (2016 TO 2020) Crash Data Location: Fabyan Parkway @ Kingsland Drive

	2016				2017			2018					2019					2020		Totals									
Crash Type	# Crashes	# K fat.	# A inj.	# B inj.	# C inj.	# Crashes	# K fat.	# A inj.	# B inj.	# C inj.	# Crashes	# K fat.	# A inj.	# B inj.	# C inj.	# Crashes	# K fat.	# A inj.	# B inj.	# C inj.	# Crashes	# K fat.	# A inj.   # B inj.   # C in	i. # Crashes	% Crashes	# K fat.	# A inj.	# B inj.	# C inj.
Pedestrian																								0	0.0%	0	0	0	0
Pedalcyclist																								0	0.0%	0	0	0	0
Train																								0	0.0%	0	0	0	0
Animal											1													1	10.0%	0	0	0	0
Overturned																								0	0.0%	0	0	0	0
Fixed Object																								0	0.0%	0	0	0	0
Other Object																								0	0.0%	0	0	0	0
Other Noncollision																								0	0.0%	0	0	0	0
Parked Motor Vehicle																								0	0.0%	0	0	0	0
Turning						1			1															1	10.0%	0	0	1	0
Rear End	1					1				1	1				1	1					1			5	50.0%	0	0	0	2
Sideswipe Same Direction						1					1										1			3	30.0%	0	0	0	0
Sideswipe Opposite Direction																								0	0.0%	0	0	0	0
Head On																								0	0.0%	0	0	0	0
Angle																								0	0.0%	0	0	0	0
Other																								0	0.0%	0	0	0	0
Total	1	0	0	0	0	3	0	0	1	1	3	0	0	0	1	1	0	0	0	0	2	0	0 0 0	10	100.0%	0	0	1	2

# Crashes w/Injuries	2016	2017	2018	2019	2020	Total
# Crashes with K fatalities						0
# Crashes with A injuries						0
# Crashes with B injuries		1				1
# Crashes with C injuries		1	1			2
Totals	0	2	1	0	0	3

Note: Crashes documenting multiple types of injuries are counted only once and in the more serious injury category.

Time of Crash	2016	2017	2018	2019	2020	Total
Darkness						0
Darkness, Lighted Road						0
Dawn/Dusk	1	2	2	1	1	7
Daylight		1	1		1	3
Unknown						0
Totals	1	3	3	1	2	10

Roadway Condition	2016	2017	2018	2019	2020	Total
# Crashes with wet pavement			2			2
# Crashes with snow/slush/ice						0
# Crashes with dry pavement	1	3	1	1	2	8
# Unknown Condition						0
Totals	1	3	3	1	2	10

Data Summary: _	Crash Data from (2016 TO 2020)
Project:	Project Name: Settler's and Kingsland Feasibility Study
Limits:	Crash Data Location: Fabyan Parkway @ Kingsland Drive
County:	Kane County
P Number:	

Time of Day	2016	2017	2018	2019	2020	Total
Midnight - 3:59 am						0
4:00 - 7:59 am	1	1	1	1	1	5
8:00 - 11:59 am						0
Noon - 3:59 pm		1	1		1	3
4:00 - 7:59 pm		1	1			2
8:00 - 11:59 pm						0
Totals	1	3	3	1	2	10



ATTACHMENT 6: Estimate of Probable Cost

#### Kane County Division of Transportation

Settler's Hill and Kingsland Drive Feasibility Study

Engineer's Opinion of Probable Cost - Alternative B Pavement Widening and Resurfacing Improvement

Finall Construction Cost \$762,015.50

A)	Surface Maintenance and Capacity Improvements	UN	UNIT COST		TED QUANTITY	Overall Cost	Assumptions
1)	HMA Binder (1.5")	\$100.00	Tons	256	Tons	\$25,600.00	The HMA Binder is assumed to be 1.5" thick and used <b>ONLY</b> for construction over the proposed pavement surface area widening.
2)	Concrete Base Course (6")	\$46.50	Square Yard	3,046	Square Yard	\$141,639.00	The PCC Concrete Base course is assumed to be 6" thick and used for <b>ONLY</b> construction over the proposed pavement surface area <u>widening</u> .
3)	Surface Course (1.5")	\$103.00	Tons	1,434	Tons	\$147,702.00	The Surface Course is assumed to be 1.5" thick and used for construction over the entirety of proposed pavement surface area.
4)	Aggregate Subbase (6")	\$9.00	Square Yard	4,436	Square Yard	\$39,924.00	The Aggregate Subbase is assumed to be 6" thick and used <b>ONLY</b> for construction over the proposed pavement surface area <u>widening</u> .
5)	Milling (1.5")	\$9.00	Square Yard	14,021	Square Yard	\$126,189.00	The Surface Course Milling is assumed to be 1.5" thick and used for construction over the entirety of existing payement surface area.
6)	Combination Concrete Concrete Curb and Gutter B-6.24	\$30.00	Foot	4841.818	Foot	\$145,254.54	Source of Unit Cost flexure flow website
7)	Combination Curb and Gutter Removal	\$7.50	Foot	4841.818	Foot	\$36,313.64	The Combination Curb and Gutter Removal Quantity is assumed to be the same as the proposed Concrete Concrete Curb and Gutter Quantity for this high level cost estimate
				Construe	ction Cost Estimate Contingency 15%	\$662,622.18 \$99,393.33	15% Contingency is assumed based on the design being a conceptual design

Note 1: The driveway and approach pavement widening surfaces are assumed to be constructed out of the same materials (base, subbase, aggregates etc.)



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